

# GENERAL NOTES

GOVERNMENT CODE OHIO BUILDING CODE

DESIGN FLOOR LIVE LOADS:	SEISMIC DESIGN DATA:
SALES AREA: 100 PSF	SEISMIC USE GROUP: 1
MEZZANINE FLOOR: 125 PSF	SEISMIC IMPORTANCE FACTOR (I <sub>p</sub> ): 1.0
DESIGN ROOF SNOW LOADS:	WIND DESIGN RESPONSE ACCELERATIONS:
GROUND SNOW LOAD (S <sub>g</sub> ) = 20 PSF	S <sub>s</sub> = 0.095
SNOW EXPOSURE FACTOR (C <sub>e</sub> ) = 1.0	S <sub>s</sub> = 0.034
SNOW LOAD IMPORTANCE FACTOR (I <sub>s</sub> ) = 1.0	SPECTRAL RESPONSE COEFFICIENTS:
THERMAL FACTOR (C <sub>t</sub> ) = 1.1	S <sub>s1</sub> = 0.064
SNOW LOAD (S <sub>s</sub> ) = 20.4 PSF	S <sub>s2</sub> = 0.054
(INCLUDES 2.4 PSF RAIN-ON-SNOW	SEISMIC DESIGN CATEGORY: A
LOADS)	SEISMIC DESIGN CATEGORY: A
FLAT ROOF SNOW LOAD (P <sub>f</sub> ) = 20.4 PSF	ODINARY REIN. AND UNREIN. MASONRY SHEAR WALLS
(INCLUDES 2.4 PSF RAIN-ON-SNOW	DESIGN BASE SHEAR (V=Q <sub>s</sub> W):
LOADS)	SLEDS ADDITION = 1.0*
DRIFTING PER ASCE 7-05	WIND EXPOSURE CATEGORY = C
MINIMUM ROOF DESIGN LIVE LOAD = 20 PSF	WIND LOAD IMPORTANCE FACTOR (I <sub>w</sub> ) = 1.0
ROOF DESIGN DEAD LOAD = 18 PSF	WIND PRESSURE COEFFICIENT (C <sub>d</sub> ): 0.048
WIND DESIGN FACTORS:	SEISMIC RESPONSE COEFFICIENT FACTOR: 2
BASIC WIND SPEED = 90 mph	SEISMIC RESPONSE COEFFICIENT FACTOR: 2
WIND EXPOSURE CATEGORY = C	SEISMIC RESPONSE COEFFICIENT FACTOR: 2
WIND LOAD IMPORTANCE FACTOR (I <sub>w</sub> ) = 1.0	ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
WIND PRESSURE COEFFICIENT (C <sub>d</sub> ): 0.048	SEISMIC RESPONSE COEFFICIENT FACTOR: 2
WIND PRESSURE COEFFICIENT FACTOR: 2	ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
WIND PRESSURE COEFFICIENT FACTOR: 2	ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE



SHOULD ANY OF THE GENERAL NOTES CONFLICT WITH THE ARCHITECT'S INSTRUCTIONS OR THE SPECIFICATIONS, THE SPECIFICATIONS SHALL GOVERN.

PARTS OF THE STRUCTURE AFFECTED BY OPENINGS RELATED TO MECHANICAL REQUIREMENTS ARE SHOWN FOR BUILDING PURPOSES ONLY. THE GENERAL CONTRACTOR SHALL VERIFY THESE CONDITIONS WITH MECHANICAL AND OTHER TRADES AND INCLUDE ANY ADDITIONAL COSTS IN BASE BID.

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES OF CONSTRUCTION AND TO VERIFY THAT THE GENERAL CONTRACTOR'S PROPOSED SEQUENCES INCLUDE THE ADDITION OF ANY NECESSARY SHORING, SHEETING, TEMPORARY BRACING, Girts OR PROTECT. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

## FABRICATION AND PLACEMENT SUBMITTALS

THE CONTRACTOR WILL SUBMIT THE DOCUMENTS TO THE ARCHITECT FOR REVIEW OF CONFORMANCE WITH THE PLANS AND SPECIFICATIONS 30 DAYS BEFORE THAT REQUIRED FOR ORDERING OR INSTALLATION AT THE JOB SITE.

THESE DRAWINGS SHALL BE CHECKED AND COORDINATED WITH OTHER MATERIALS AND CONTRACTS BY THE GENERAL CONTRACTOR AND BEAR CHECKER'S INITIALS BEFORE BEING SUBMITTED TO THE ARCHITECT FOR APPROVAL.

PROPRIETARY PRODUCTS REQUIRE PHYSICAL PROPERTIES OF MATERIAL AND ANY PREPARATIONS FOR INSTALLATION OR APPLICATION METHODS.

CUSTOM FABRICATED ITEMS REQUIRE FABRICATION AND PLACEMENT DRAWINGS OF ALL MATERIALS. THESE DRAWINGS MUST INCLUDE PLANS AND DETAILS OF EACH MATERIAL WITH ANY INSTALLATION SEQUENCE. CALCULATIONS ARE NECESSARY FOR ALL STRUCTURAL MEMBERS WITH DETAILS, LOCATIONS, AND LOADS ON ANY CONNECTIONS TO OTHER BUILDING MATERIALS.

ALL CONNECTIONS, PLUMBING, AND OPERATING EQUIPMENT REQUIRE THE LOCATIONS OF ANY OPENINGS AND CONNECTION METHODS TO ANY STRUCTURAL PARTS OF THE BUILDING.

## FOUNDATIONS

ELEVATIONS AND DETAILS OF THE EXISTING FOOTINGS ARE BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME OF DESIGN. AS PART OF THE CONSTRUCTION, THE CONTRACTOR SHALL VERIFY THE EXISTING FOUNDATIONS AND MAKE ANY NECESSARY CORRECTIONS. ALL NEW WORK SHALL BE PERFORMED IN ACCORDANCE WITH ACTUAL SITE CONDITIONS.

DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL FLOOR SLABS ARE IN PLACE OR PERMISSION IS OBTAINED FROM THE ARCHITECT.

ALL FOUNDATIONS ARE TO REST ON FIRM UNDISTURBED SOIL. OF BEARING CAPACITY NOTED, REGARDLESS OF ELEVATIONS SHOWN ON DRAWINGS. WHERE ROCK OR SHALE IS THE BEARING MATERIAL, PROVIDE 6" DEEP FRENCH DRAIN.

ALL SOIL SURROUNDING AND UNDER FOOTINGS SHALL BE PROTECTED FROM FROST ACTION AND FREEZING DURING THE COURSE OF CONSTRUCTION.

BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE AT LEAST 3'-6" BELOW FINISHED GRADE.

STEP BOTTOM OF FOOTING, WHERE REQUIRED, AT A RATE OF 1 UNIT VERTICAL TO 2 UNITS HORIZONTAL. MAXIMUM VERTICAL STEP 2'-0".

WHERE FOOTINGS ARE IN CLOSE PROXIMITY OF SEWERS, BOTTOM OF FOOTINGS SHALL BE AT LEAST 8" BELOW INVERT ELEVATION OF SEWERS.

KEEP FOUNDATION EXCAVATIONS FREE OF WATER AT ALL TIMES.

USE LEAN CONCRETE (f'<sub>c</sub> = 1500 PSI) FOR OVER-EXCAVATION OF FOOTINGS AND REPLACEMENT OF WEAKENED SOIL.

ALL NEW FOOTINGS ARE DESIGNED BASED ON AN ALLOWABLE BEARING CAPACITY OF 2500 PSF AS PER RECORD DOCUMENTS.

## SLAB ON GRADE

PROVIDE 1'-0" WIDE THICKENED SLAB WITH (2)-#4 CONTINUOUS UNDER ALL NON-BEARING MASONRY WALLS. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS.

CONTROL JOINTS SHALL BE CREATED BY SAW CUTS, 1/8" WIDE TO A DEPTH EQUAL TO 1/5 OF THE SLAB THICKNESS, LOCATED ON COLUMN CENTRILINES AND AT 12'-0" O.C. (PLUS OR MINUS 4'-0") IN EACH DIRECTION, UNLESS NOTED OTHERWISE.

CONSTRUCTION JOINTS SHALL BE KEPT AND LOCATED AT CONTROL JOINT INTERVALS.

## CONCRETE

CONCRETE SHALL DEVELOP A COMPRESSIVE STRENGTH IN 28 DAYS FOR FOUNDATIONS: 3000 PSI FOR FOUNDATIONS: 4000 PSI ALL OTHER CONCRETE: 4000 PSI

CONCRETE WORK, DETAILING, FABRICATION, AND ERECTION OF STEEL REINFORCING BARS SHALL CONFORM TO THE LATEST A.C.I. SPECS. AND DETAILING MANUAL.

CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED IN CONCRETE. PROVIDE 4" MINIMUM COVER FOR ALL STRUCTURAL STEEL BELOW GRADE.

REINFORCING STEEL SHALL BE ASTM GRADE 60 (F<sub>y</sub>=60 KSI).

WELDED WIRE FABRIC SHALL CONFORM TO ASTM-185.

NO TACK WELDING OF REINFORCING STEEL WILL BE PERMITTED.

ALL LAP SPICES SHALL BE CLASS C, UNLESS NOTED OTHERWISE.

PROVIDE CORNER DOWELS IN REINFORCED CONCRETE WALLS AND TRENCH FOOTINGS OF SAME SIZE AND SPACING AS HORIZONTAL REINFORCING. DOWELS SHALL HAVE A CLASS C LAP WITH HORIZONTAL REINFORCING IN EACH DIRECTION.

ALL CONCRETE ACCESSORIES EXPOSED TO WEATHER SHALL BE CORROSION RESISTANT.

WHERE MESH REINFORCEMENT SHALL LAP A MINIMUM OF 1'-0" AT SIDES AND ENDS AND AND BE WIRED TOGETHER.

REINFORCING STEEL SHOP DRAWINGS SHALL INDICATE THE SEQUENCE IN WHICH LAYERS OF CROSSING REINFORCING SHOULD BE PLACED IN ORDER TO PRODUCE THE CORRECT OUTERMOST LAYERS AS INDICATED ON THE DRAWINGS.

## EXISTING CONDITIONS

BEFORE TO START OF CONSTRUCTION, CONTRACTOR SHALL VERIFY ALL EXISTING CONSTRUCTION, PARTS OF THE STRUCTURE, AND FOUNDATIONS. ANY DEFICIENCIES SHALL BE NOTED WITH DESIGN DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT IMMEDIATELY.

CONTRACTOR IS RESPONSIBLE FOR PROTECTING ALL EXISTING CONSTRUCTION TO REMAIN. ANY DAMAGE CAUSED BY THE CONTRACTOR SHALL BE REPAIRED OR REPLACED TO THE SATISFACTION OF THE OWNER AND ARCHITECT AT NO COST TO THE OWNER.

## MASONRY

MATERIALS:

HOLLOW LOAD BEARING UNITS	(GRADE N-11)	ASTM C	90
SOLID LOAD BEARING UNITS	(GRADE N-11)	ASTM C	145
CONCRETE BRICK	(GRADE N-11)	ASTM C	55
MORTAR	(TYPE S)	ASTM C	478
GROUT FOR REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.			

WALLS SHOWN ON STRUCTURAL DRAWINGS ARE FOR CLARITY ONLY. SEE ARCHITECTURAL DRAWINGS FOR COMPOSITION OF MASONRY WALLS EXCEPT PIERS.

ALL HOLLOW UNITS SHALL BE Laid WITH FULL MORTAR COVER ON HORIZONTAL AND VERTICAL EDGES OF FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES OF BEARING WALLS. PIERS, COLUMNS AND PLUSTERS, STARTING COURSE ON FOOTINGS ADJACENT TO CELLS OR CAVITIES SHALL BE FILLED WITH GROUT.

SOLID UNITS SHALL BE Laid WITH FULL HEAD AND BED JOINTS.

COLLAR (VERTICAL LONGITUDINAL) JOINTS BETWEEN THE FACING AND BACKING WTHERS IN WALLS SHALL BE COMPLETELY FILLED WITH MORTAR OR GROUT WHICH SHALL BE WORKED IN WITH A TROWEL.

MORTAR PROTRUSIONS EXTENDING INTO CELLS OR CAVITIES TO BE GROUTED SHALL BE REMOVED IN MASONRY WALLS. NO CHASES, RISERS, CONDURTS, OR TOOTHING OF MASONRY SHALL OCCUR WITHIN 17" OF CENTRELINE OF BEAM BEARING OR LOAD CONCENTRATION.

USE 2 COURSES (16") OF SOLID MASONRY BELOW EACH BEAM OR UNTEL BEARING.

USE 1 COURSE (8") OF SOLID MASONRY BELOW EACH STEEL JOIST BEARING UNLESS NOTED OTHERWISE. BOLTS OR ANCHORS SHALL BE EMBEDDED IN GROUT.

INTERSECTING LOAD BEARING WALLS SHALL BE TIED TOGETHER IN MASONRY BOND UNLESS NOTED OTHERWISE.

WALLS WILL HAVE EXTRA HEAVY TRUSS-TYPE STEEL WIRE MORTAR JOINT REINFORCING AT 16" O.C. VERTICAL SPACING.

CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED IN MORTAR OR GROUT.

PIERS:

IF NOT SPECIFICALLY INDICATED, CONSTRUCT PIERS USING SAME MASONRY AS THAT IN WALL. PIERS DESIGNATED SHIP SHALL BE CONSTRUCTED USING CONCRETE BUILDING BRICK CONFORMING TO ASTM C 55 GRADE N-11.

BOND PIERS INTO ADJACENT WALLS USING PIER MASONRY MATERIAL FOR TOOTHING.

PROVIDE 2 ADDITIONAL #5 BARS AT EACH SIDE OF ALL OPENINGS AS WELL AS ABOVE AND BELOW EACH OPENING AND AT THE ENDS OF WALLS. BARS TO EXTEND A MINIMUM OF 24" PAST THE EDGE OF OPENINGS.

## METAL DECK

DECK SHALL CONFORM TO ALL REQUIREMENTS OF "BASIC DESIGN SPECIFICATIONS" AS ADOPTED BY THE S.D.I.

METAL ROOF DECK SHALL BE 22 GADE, 1/2" WIDE RIB DECK, PAINTED EQUIVALENT TO VULCRAPT 1.9B22.

ATTACHMENT: METAL DECK SHALL BE WELDED TO ALL SUPPORTING STEEL WITH PUDDLE WELDS (5/8" MINIMUM) AT 12" O.C. (3" PER SHEET MINIMUM).

SIDE LAP CONNECTIONS SHALL BE MADE WITH #12 SELF-TAPPING SCREWS AT A MAXIMUM OF 3'-0" O.C.

## PRECAST CONCRETE FLOOR PLANK

MEZZANINE FLOOR PLANK SHALL BE 8" HOLLOWCORE PRESTRESSED CONCRETE SLAB UNITS - SPANCRETE OR EQUAL.

DESIGN LOADS: LIVE LOAD..... 125 psf SUPERIMPOSED DEAD LOAD OF TOPPING)..... 20 psf (DOES NOT INCLUDE WEIGHT OF TOPPING)

CONTRACTOR SHALL PROVIDE SHOP DRAWINGS AND CALCULATIONS CERTIFIED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF OHIO.

## STRUCTURAL STEEL

MATERIALS:

STRUCTURAL STEEL:	ASTM A 992 (F <sub>y</sub> =50 ksi)
W SHAPES:	ASTM A 36 (F <sub>y</sub> =36 ksi)
ANGLES, CHANNELS, PLATES:	ASTM A 500, GRADE B
HSS/TUBE SECTIONS:	ASTM A 500, GRADE B
ANCHOR BOLTS:	ASTM A 305
WELDING ELECTRODE:	AMS E 70XX

DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST A.I.S.C. SPECIFICATIONS.

CONNECTIONS:

NON-COMPOSITE MEMBERS: DESIGN CONNECTIONS FOR FULL STRENGTH OF MEMBER FOR SPAN PER A.I.S.C. BEAM LOAD TABLES UNLESS NOTED OTHERWISE.

SHOP CONNECTIONS SHALL BE BOLTED (3/4" x 325 BOLTS) OR WELDED UNLESS NOTED OTHERWISE. FIELD CONNECTIONS SHALL UTILIZE 3/4" x 325 SC BOLTS.

ONE SIDED CONNECTIONS WILL NOT BE PERMITTED UNLESS DETAILED ON THE DRAWINGS.

BOTTOMS OF ALL CONNECTIONS SHALL BE AT LEAST 6" ABOVE FINISHED CEILING.

BEAMS BEARING ON MASONRY SHALL HAVE ANGLE WALL ANCHORS AS SHOWN IN THE A.I.S.C. MANUAL EXCEPT ANGLES SHALL BE WELDED TO THE BEAMS OR SHALL BE WELDED TO THE BEARING PLATES.

ALL LOOSE BEAM UNTELS SHALL HAVE 8" MINIMUM BEARING UNLESS NOTED OTHERWISE.

LOOSE UNTELS OVER DOORS, WINDOWS, DUCTS, AND MISCELLANEOUS OPENINGS TO BE USED FOR EACH 4" OF MASONRY WALL THICKNESS UNLESS NOTED OTHERWISE:

- (1) 2x 3 1/2" x 4" x 5/16" FOR SPANS UP TO 4'-0" WITH 6" BEARING EACH END.
- (1) 2x 3 1/2" x 5" x 5/16" FOR SPANS UP TO 5'-0" WITH 6" BEARING EACH END.
- (1) 2x 3 1/2" x 6" x 5/16" FOR SPANS UP TO 6'-6" WITH 8" BEARING EACH END.

SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MASONRY OPENINGS.

PROVIDE 9/16" ROUND HOLES AS REQUIRED BY ARCHITECTURAL DRAWINGS FOR WOOD NAILERS.

ALL STRUCTURAL STEEL BEAMS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE STRAP ANCHORS AT 2'-8" O.C. OR APPROVED EQUAL, 1 1/2" x 1/8"

PROVIDE 1/16" DRAW FOR EACH 10'-0" LENGTH OF BRACING.

SUBSTITUTION OF MEMBERS IS PERMITTED TO EQUIVALENT DELIVERY. SUBSTITUTE MEMBERS SHALL BE THE SAME NOMINAL DEPTH AND HAVE A WEIGHT PER FOOT GREATER THAN THE MEMBER INDICATED. ALL WELDING SHALL CONFORM TO THE CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION OF THE A.W.S. AND BE PERFORMED BY A CERTIFIED WELDER IN ACCORDANCE WITH A.W.S. STANDARDS.

SEQUENCE OF PLACING WELDS SHALL BE SUCH AS TO AVOID DISTORTION OF MEMBERS. NO WELDING OR FINAL BOLTING SHALL BE DONE UNTIL STRUCTURE HAS BEEN PROPERLY ALIGNED.

## STEEL JOISTS

STEEL JOISTS SHALL CONFORM TO AND BE ERECTED IN ACCORDANCE WITH THE LATEST S.A.I. SPECIFICATIONS.

EXTEND JOIST BEARING END 1" MINIMUM PAST CENTRELINE OF SUPPORTING BEAM WHERE POSSIBLE.

ADJACENT JOISTS OF SAME DEPTH ARE TO HAVE WEB MEMBERS IN LINE TO PERMIT PASSAGE OF MECHANICAL DUCT WORK.

JOIST EXTENSIONS, WHETHER CANTILEVERED OR SUPPORTED, SHALL PROVIDE A MINIMUM LOAD SUSTAINING CAPACITY EQUAL TO THE JOIST.

ALL JOIST BRACING SHALL CONFORM TO S.A.I. STANDARDS AND BE IN ACCORDANCE WITH O.S.H.A. REGULATIONS.

## COLD-FORMED METAL FRAMING

MATERIALS: STUDS AND METAL LIGHTS SHOWN ON THE CONTRACT DOCUMENTS ARE MANUFACTURED TO DESIGNSPECIFIC REQUIREMENTS ONLY. AS REQUESTED, PROVIDING A 6-INCH DEEP STUD WITH 1-5/8" FLANGES - 18 GA (43 mils) THICK.

MEMBER PROPERTIES USED FOR DESIGN ARE BASED UPON MEMBER INFORMATION FROM DETRICH INDUSTRIES (F<sub>y</sub> = 50 ksi). SUPPLIED MEMBERS MUST MEET THE SAME DETRICH SECTION PROPERTIES.

ALL TRACKS AND ACCESSORIES: F<sub>y</sub> = 50 ksi MINIMUM.

SPECIFICATIONS: WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AWS DESIGN, FABRICATION, AND ERECTION TO BE COVERED BY LATEST REVISIONS OF A.S.I. SPECIFICATION OF THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.

STRUCTURAL WELDING CODE: AWS D3 OF THE AMERICAN WELDING SOCIETY.

CONNECTIONS:

FIELD CONNECTIONS MAY BE EITHER WELDED OR SCREWED, EXCEPT AS SPECIFICALLY DETAILD.

ALL CONNECTIONS SHALL BE MADE WITH A MINIMUM OF (4) MECHANICAL FASTENERS.

FINISH:

ALL MATERIAL TO BE GALVANIZED COATED IN ACCORDANCE WITH ASTM A525 C-60. TOUCH-UP FIELD WELDS WITH ZINC RICH PAINT.

MISCELLANEOUS:

ALL FIELD CUTTING TO BE PERFORMED WITH A SAW.

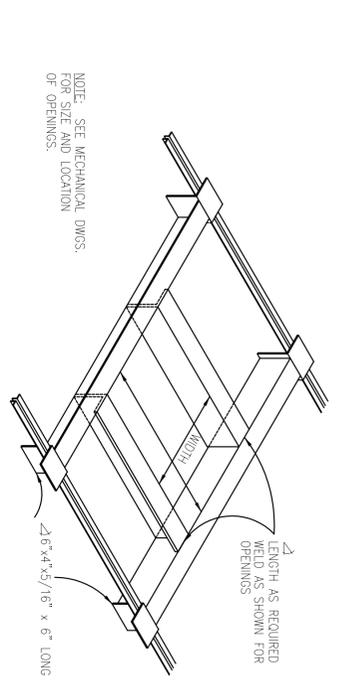
WELD SIZE TO BE 3/32" OR AS NOTED WITH AWS TYP. 6013 OR 7014 R00.

EXCEPT AS NOTED OTHERWISE, MECHANICAL FASTENERS TO BE SELF TAPPING #10 SCREWS AS MANUFACTURED BY BILUDEK, INC.

TRACKS TO BE SECURELY ANCHORED TO SUPPORTING STRUCTURE WITH WELD OR SCREW AT EACH SIDE OF TRACKS.

PROVIDE BRACING AT 48" O.C. MAX. FOR ALL STUD WALLS AND TRUSS MEMBERS. UNLESS NOTED OTHERWISE.

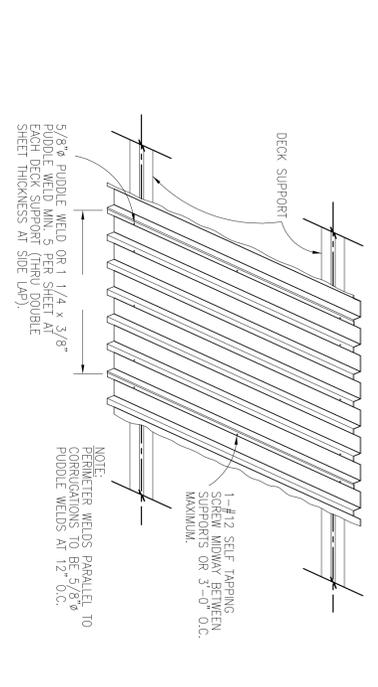
END BRACING OR CONTINUOUS TRACK IS TO BE PROVIDED WHERE JOIST ENDS ARE NOT OTHERWISE RESTRAINED FROM ROTATION.



## TYPICAL ROOF OPENING DETAIL

BURGLAR BARS MAY BE REQUIRED. VERIFY WITH MECHANICAL AND ARCHITECTURAL DRAWINGS.

OPENINGS MAY EXTEND INTO ADJACENT BAYS. PROVIDE SAME DETAIL, VERIFY UNIT CURB SIZES WITH MECHANICAL CONTRACTOR PRIOR TO FABRICATION.



## TYPICAL DECK ATTACHMENT DETAIL

NOTE: INTERFERING WELDS PARALLEL TO CONNECTIONS TO BE 5/8" PUDDLE WELDS AT 12" O.C.

DECK ATTACHMENT NOTES:

TIGHT DECK TO SUPPORT CONTACT SHALL BE MAINTAINED AT ALL WELD LOCATIONS.

WELDING ROD APPEARANCE AND BURR OFF RATE SHALL BE DETERMINED BY FIELD TESTING AS FOLLOWS:

TWO WELDS AT 6"OC. LEVERAGE SHOULD BE APPLIED TO THE PANEL. YIELDING SHOULD NOT BE NOTED AROUND WELD AND SUDDEN SEPARATION SHOULD NOT OCCUR ON THE CONTACT PLANE. SHOULD EITHER OCCUR, WELDING TIME AND/OR WELDING AMPERAGE SHALL BE ADJUSTED UNTIL ADEQUATE WELDING IS ACCOMPLISHED.

SET NOTES FOR SIZE AND SPACING.

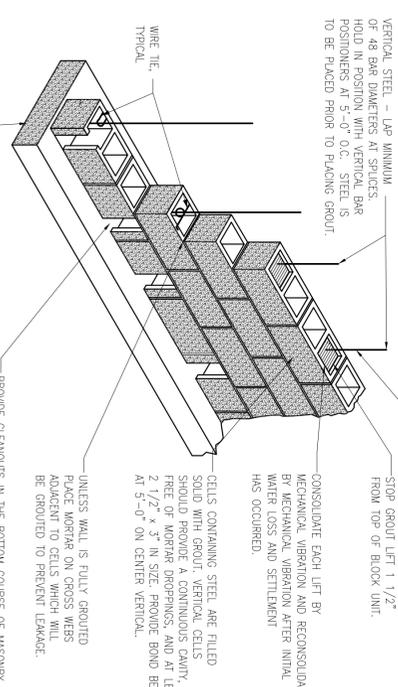
STOP GROUT LEFT 1 1/2" FROM TOP OF BLOCK UNIT.

CONSOLIDATE EACH LIFT BY MECHANICAL VIBRATION AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED.

CELLS CONTAINING STEEL ARE FILLED WITH GROUT. VERTICAL CELLS SHOULD PROVIDE A CONTINUOUS CURTAIN FREE OF MORTAR DRIPPINGS, AND AT LEAST 2 1/2" x 3" STEEL PROVIDE BOND BEAM AT 5'-0" ON CENTER VERTICAL.

UNLESS WALL IS FULLY GROUTED, PLACE MORTAR ON CROSS WEBS ADJACENT TO CELLS WHICH WILL BE GROUTED TO PREVENT LEAKAGE.

PROVIDE CLEANOUTS IN THE BOTTOM COURSE OF MASONRY IN EACH GROUT POUR AT EACH VERTICAL BAR. THE SIZE OF THE CLEAN OUT OPENINGS SHALL BE OF SUFFICIENT SIZE TO PERMIT REMOVAL OF DEBRIS. INSPECT THE MINIMUM OPENING AND VERTICAL REINFORCEMENT PRIOR TO PROVIDING CLOSURE TO RESIST GROUT PRESSURE.



## TYPICAL REINFORCED MASONRY CONSTRUCTION DETAIL

PROVIDE CLEANOUTS IN THE BOTTOM COURSE OF MASONRY IN EACH GROUT POUR AT EACH VERTICAL BAR. THE SIZE OF THE CLEAN OUT OPENINGS SHALL BE OF SUFFICIENT SIZE TO PERMIT REMOVAL OF DEBRIS. INSPECT THE MINIMUM OPENING AND VERTICAL REINFORCEMENT PRIOR TO PROVIDING CLOSURE TO RESIST GROUT PRESSURE.

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